

Review On The Effect Of Shear Connectors On Composite Deck Slabs

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Abstract— This paper presents a review on the effect of shear connectors on composite deck slabs. Composite deck slabs consist of profile deck sheet and concrete. Several researchers have been studying the behaviour of composite slab but due to its complex behaviour yet it is not completely understood. The behaviour of composite slab directly depends on the deformability and contact strength. Here, some important literature reviews regarding composite slab behavior incorporating different profiles were discussed.

Keywords—Composite Deck Slab, Shear Connectors, Modern Constructions, Profiled Steel Deck.

1. INTRODUCTION

Composite steel-concrete structures are used widely in modern bridge and building construction. The fact that each material (steel or concrete) is used to take advantage of its best attributes makes composite steel-concrete construction very efficient and economical. However, the real attraction of composite construction is based on having an efficient connection of the steel to the concrete, and it is this connection that allows a transfer of forces and gives composite members their unique behavior. Shear connectors are provided to transfer load from composite steel deck to the steel beam. Shear flow forces have to be resisted by the shear flow strengths of the mechanical shear connectors that are used to tie the concrete component to the steel component. The shearing force at composite beams with profiled sheet is not introduced directly through the base of the shear connector, but is transferred onto the shank of the shear connector. The increase of load produces crushing of concrete in front of the shear connector and transfer of shearing force exclusively via bending.

2. TYPES OF STEEL DECK

2.1 Box Profile 34/1000 Sheets (for roofing and cladding):

Cladco Profiles 34/1000 trapezoidal sometimes referred to as box profile single skin sheet gives maximum sheet strength with 7 rigid profiles of 166mm pitch across the sheet width and 34mm profile height. It's made from wider coated steel strip to give a secure weather proof lap and gives 1000mm actual width cover when lapped as shown in figure 1.

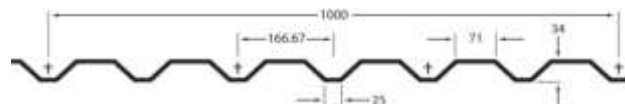


Fig. 1.Box Profile 34/1000

2.2 Box Profile 32/1000 Sheets (for roofing and cladding):

Cladco Profiles 32/1000 trapezoidal sometimes referred to as box profile single skin sheet gives robust sheet strength with 6 profiles of 200mm pitch across the sheet width and 32mm profile height. Some users prefer the appearance of this profile as it has lower and less profiles. It's made from coated steel strip and gives 1000mm actual width cover when lapped as shown in figure 2.

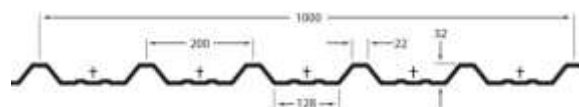


Fig. 2. Box Profile 32/1000

2.3 Corrugated Profile 13/3 (for roofing and cladding):

Cladco Profiles traditional 13/3 'corrugated iron' profile. 13 corrugations of 3 inch/76mm round corrugation gives 3ft 3inch/990mm sheet cover when lapped as shown in figure 3.

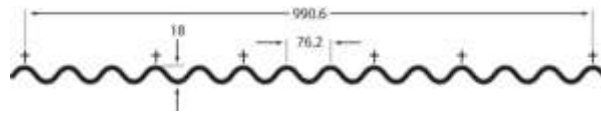


Fig. 3. Profile 13/3

3. PREVIOUS STUDIES

Wang Yu Hang and NIE JianGua, 2015 [1], studied theoretical model for ultimate load carrying capacity of composite slabs cast with profiled steel sheets considering the relation between ultimate moment carrying capacity of the section and tensile force resisted by the profile deck sheet. Analytical study was carried out by considering two span continuous slabs subjected to one-point load, two-point load and uniformly distributed load. Based on the developed stress diagram an equation was derived for all these three cases. The study was also carried out by experimentally testing the specimens considering same loading conditions to validate the theoretical work. The investigators state that the results of other, analytical and experimental study are found to be similar. It is recommended that though the M-K method is convenient for designing the composite slabs, it cannot be used for direct practical approach it is repetitive in nature and this tends to uneconomical design. Hence, in this paper researcher suggested an alternative method for calculating ultimate load carrying capacity of the composite slab known as simplified calculation method. The method is based on the dimension less parameters considering both flexure and longitudinal failure but can be used only in case of uniformly distributed loading condition.

M. A. Bradford et al, 2011 [2], studied the long term effect due to shrinkage of concrete which will result in initial indirect strain in composite slab as creep in concrete is observed in initial stage after casting. The researcher derived anew for deflection and stress equation based on virtual work method by considering the partial shear interaction between concrete and steel sheet subjected to shrinkage straining. It was observed in experimental study that quantification of the shrinkage and partial interaction is seen in the uncracked area as it allows strain to be transferred through the depth of the slab due to difference in corrugation depths of the profile deck sheet. The results indicated that forecast response for the slab was complex as the combination of the shrinkage and partial interaction gave rise to the small concrete stresses which should be analytically studied for giving the application based structural mechanism.

M.E. A-H Eldib, 2009 [3], studied the behavior of two-way composite slab using finite element analysis in COSMOS/M26 software. The researcher compared one way and two way slabs using nonlinear material properties and full-scale models with varying parameters such as slenderness ratio and slab aspect ratio. The analytical results were compared in respect of ultimate load carrying capacity, reaction distribution and deflection of composite slab. The side perpendicular to the ribs of profile deck sheet of a composite slab is usually the weaker side, whereas the side along the ribs is stronger. Hence, the study of behaviour of composite deck slab along the weaker side helps in enhancing the ultimate load carrying capacity of the slab. The investigators report that, though the slab is a two way slab its failure at ultimate load resembles as that of one-way slab, because value of load factor at serviceability limit and percentage of reaction is greater on weak side. However, Shear connectors and cold steel straps fixed at bottom steel deck prevents or say reduces the deflection and considerably increase the strength of composite slab.

Redzuan A. and W. Samuel, 2008 [4], studied new procedure to estimate the horizontal shear in which slab slenderness ratio is considered to be the predominant factor affecting the horizontal shear force. The researcher derived new equation based on force equilibrium method for deriving the relation between shear bond stress and end slip. The justification of the derived relation was also carried out with the conventional partial interaction method. The relation between shear bond stress and end slip was similar by both the methods. To validate the results of the analytical work experimentation carried out for bending test with two-point load bending test and slip observed was larger in compact slab than in slender slab. For full interaction slab shear bond and end slip curve was plotted along the vertical axis i.e. end slip does not occur for the slab with full interaction. While in partial interaction the shear bond is unchangeable. The researcher concluded that the shear bond property changes with change in combination of slenderness or geometry or material. For particular case for same material and geometry the only property that affects shear bond is slenderness. So most important property horizontal shear bond stress–end slip relation can be obtained from force equilibrium method which is very important for numerical analysis.

Emad El Dardiry and Tianjim Ji, 2006 [5] studied the dynamic behaviour of composite slab using isotropic and orthotropic plate model. The properties of section were kept same in both directions for isotropic model but different in orthotropic model. The analytical results of the study indicate that the mode shapes checked for each panel are either concave or convex though the

complex behaviour is taken in consideration. However, the isotropic model proves to be slightly better than orthotropic model. In case of effect of boundary condition, orthotropic model proves to be stiffer and its effective thickness remains constant, while good performance was noted for isotropic plate where thickness varies from 5 to 6%. In case of load combination less error was noted for isotropic plate i.e. 4 % and for orthotropic plate 10% was noted. Now eccentricities are considered to find effect on natural frequency and it was found that ratio of natural frequency for model with and without eccentricities vary from 74% to 78%. Natural frequency is not sensitive to location of natural axis of slab and beam as far as eccentricity is considered. The composite slab contributes 16% of total stiffness of structure. The prediction of natural frequency is affected by the thickness of slab.

Marciukaitis et al; 2005 [6] studied the connection between profile sheet and concrete directly affect the overall deflection for the composite slab which directly depends on the shear stiffness due to connection. The researcher evaluated a new method to evaluate deflection of composite slab considering all the factors starting from loading to the ultimate moment. The connection between profile sheet and concrete is not rigid or stiff, but it is partially in contact with each other. The stiffness between the layer's changes with the action of external forces. As there are two different materials they will undergo different deformations under loading. Though there are shear deformations these deformations are largely governed by the difference in deformation. For small shear forces the slab behaves elastically and for larger shear forces the plastic behaviour is noted. According to elastic small deformations is noted and large deformations are seen in plastic behaviour. The equation where derived by considering two layers' composite member deflection. The researcher proved that there is no stiff bond between profile sheet and concrete. Hence partial stiffness should be considered while calculating deflection.

K. N. Lakshmikandhan et al., 2013 [7] have experimentally studied three types of mechanical connector and found that three connector schemes exhibited full shear interaction and produced a negligible slip.

Milan Veljkovic 1996, [8] Studied and investigate analysis based on interaction characteristics evaluated from small scale tests has been shown to accurately describe the behaviour of composite slabs. The interaction characteristics considered are mechanical interlocking resistance, reduction of the mechanical interlocking due to strains in the sheeting and the friction at the support. The validation is, however, limited to comparisons with full-scale test on one particular profile. The load bearing resistance of slabs increases as the loading becomes more uniform when longitudinal shear failure governs. For one example where longitudinal shear failure governs, the increase of the resistance with respect to 2-point loading is 20%, 30% and 40% for 4-point, 8-point and 16-point loading, respectively. All slabs have the same shear span. Due to a change of the failure mode, differences between the bearing resistances decrease as the span increases.

Hyeong-Yeol Kim, Youn-Ju Jeong 2010, [9] investigated the ultimate behavior of a SCC composite deck slab system with profiled steel sheeting and perfobond rib shear connectors. Eight specimens were prepared and develop composite deck slab for girder bridges that spans longer but weighs less than conventional reinforced concrete slab, which were evaluated using empirical m-k method. The ultimate strength under bending was at least 20% less than RC deck. The load carrying capacity is approx. 2.5 times greater, while concrete cracking load is 7.1 times greater with total weight 2.5% lesser.

Amar Prakash, N. Anandavalli, C. K. Madheswaran, J. Rajasankar, N. Lakshmanan 2011, [10] proposed nonlinear behavior of stud connected to steel-concrete composite girders numerically. 3D modeling through ABAQUS is analyzed. Specimen of 4 m length and width of 0.665 m with 3.8 m is simply supported. Maximum load observed was 360 kN analytical to 330 kN experiment outcome. The deflection obtained 136 mm analytical to 138 mm experiment outcome. Comparison of interface slips at three different values of deflection.

4. CONCLUSIONS

The study until now was performed on analysing and predicting the behaviour of the composite steel structure with concrete deck slab. Many researchers derived the expressions for deflection, ultimate moment, load carrying capacity and many more. Some questions regarding the behaviour of the slab for horizontal shear resistance and design requirements to achieve ductile failure. The optimization of the design for the proper combination of different corrugated profile sheets and type of mechanical interlocking to be provided is not yet completed. Though codal provisions are based on experimental results the finite element analysis of composite deck slab with nonlinear contacts between the profile, shear connectors and concrete is required to be studied for the complex composite action.

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